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# Structure of the hydraulic jump in convergent radial flows

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We are interested in the modelling of multi-dimensional turbulent hydraulic jumps in convergent radial flow. To describe the formation of intensive eddies (rollers) at the front of the hydraulic jump, a new model of shear shallow water flows is used. The governing equations form a non-conservative hyperbolic system with dissipative source terms. The structure of equations is reminiscent of generic Reynolds-averaged Euler equations for barotropic compressible turbulent flows. Two types of dissipative term are studied. The first one corresponds to a Chézy-like dissipation rate, and the second one to a standard energy dissipation rate commonly used in compressible turbulence. Both of them guarantee the positive definiteness of the Reynolds stress tensor. The equations are rewritten in polar coordinates and numerically solved by using an original splitting procedure. Numerical results for both types of dissipation are presented and qualitatively compared with the experimental works. The results show both experimentally observed phenomena (cusp formation at the front of the hydraulic jump) as well as new flow patterns (the shape of the hydraulic jump becomes a rotating square).

**Key words:** pattern formation, shallow water flows, shear waves

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## 1. Introduction

Much work has been done on the study of hydraulic jumps in a divergent radial flow (Liu & Lienhard 1993; Bush & Aristoff 2003; Ray & Bhattacharjee 2007; Kasimov 2008; Andersen, Bohr & Schnipper 2010; Teymourtash, Khavari & Passandideh-Fard 2010; Eyo, Joshua & Udoh 2011; Martens, Watanabe & Bohr 2012; Labousse & Bush 2013; Rojas, Argentina & Tirapegui 2013). Such a flow can easily be created by the vertical impact of a jet of water on a solid flat surface. After the impact, the flow spreads radially outwards and at some distance from the point of impingement an abrupt depth transition occurs (a hydraulic jump appears). Despite the radial symmetry of the initial flow and corresponding boundary conditions, the hydraulic jump is not necessarily circular: its shape can even have a polygonal structure with sharp corners (see Ellegaard *et al.* (1998, 1999), Bush, Aristoff & Hosoi (2006)).

An interesting experimental study for convergent radial flows was recently performed by Foglizzo *et al.* (2012, 2015). A radial symmetry breaking was also

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observed: under some flow conditions a rotating singular point (a cusp) appears at the front of the hydraulic jump.

A reliable mathematical model which is capable of capturing this complex phenomenon of symmetry breaking is still lacking. The aim of this article is to model such a phenomenon by using the model of shear shallow water flows (Teshukov 2007; Richard & Gavrilyuk 2012, 2013). Shear effects are necessary to describe surface rollers of the hydraulic jump. In the one-dimensional (1D) case, such a model complemented by friction terms was used for the study of travelling waves down inclined planes (roll waves) and turbulent hydraulic jumps. A strong physical adequacy of the model with the experimental observations was found (Richard & Gavrilyuk 2012, 2013; Richard 2013; Ivanova *et al.* 2017). The classical Saint-Venant equations are not able to predict these phenomena, because they do not take into account vorticity effects. The new model was able to form rollers in the hydraulic jumps through the Rankine–Hugoniot relations for the corresponding hyperbolic and conservative equations. In particular, the model was able to describe the hydraulic jump shape as well as jump toe oscillations appearing when the Froude number is larger than approximately 1.5. A natural step would be to apply this model for the description of multi-dimensional turbulent hydraulic jumps, and, in particular, to see if the model of shear shallow water flows is able to describe the breaking of radial symmetry in hydraulic jumps observed experimentally.

The governing equations are obtained by depth averaging of the multi-dimensional Euler equations (Teshukov 2007; Richard & Gavrilyuk 2012, 2013). The hypothesis of weakly sheared flows allows us to keep the second-order depth-averaged correlations in the governing equations but neglect the third-order correlations, and thus to close the governing system in the dissipationless limit.

The corresponding multi-dimensional model of shear shallow water flows is a hyperbolic system of equations which is reminiscent of generic Reynolds-averaged Euler equations for barotropic turbulent flows. The model has three families of characteristics, corresponding to the propagation of surface waves, shear waves and average flow. The main difficulty is the non-conservativity of the governing equations: for six unknowns (the fluid depth, two components of the depth-averaged horizontal velocity, and three independent components of the symmetric Reynolds stress tensor) one has only five conservation laws: conservation of mass, momentum, energy and mathematical ‘entropy’. The last one determines the evolution of the determinant of the Reynolds stress tensor. The non-conservative nature of the multi-dimensional equations represents an enormous difficulty from the mathematical and numerical point of view. The definition and computation of discontinuous solutions for non-conservative hyperbolic equations is a challenging problem. Recently, a new numerical method based on Cartesian meshes was developed for this non-conservative system (Gavrilyuk, Ivanova & Favrie 2018a). In the present work we extend this technique to polar coordinates.

Another difficulty is to introduce the dissipative terms into a model that still remains an empirical procedure. In particular, in Gavrilyuk *et al.* (2018a) we introduced dissipative terms which ensure the positive definiteness of the Reynolds stress tensor. Also, in the 1D limit we recovered the dissipation law developed in Richard & Gavrilyuk (2013). In particular, this dissipation law allowed us to describe the ‘fingering’ phenomenon (i.e. the formation of transverse waves on the front of roll waves). We will also test here another type of dissipation law used earlier in Gavrilyuk, Liapidevskii & Chesnokov (2016) to study surface wave propagation in shear flows.

Different dissipation laws give us different flow patterns. The first type of dissipation law gives us hydraulic jumps with the formation of a rotating cusp at the front of the hydraulic jump, as in the experiments by Foglizzo *et al.* (2012). The second type of dissipation law gives us an unusual square-like hydraulic jump. This square-like jump has no sharp corners and rotates at a constant velocity. This is different from the sharp corner structures of hydraulic jumps obtained in divergent radial flows (Ellegaard *et al.* 1998, 1999).

The structure of the article is organized as follows. In §2 the ‘shear shallow water’ equations are presented. Two types of dissipative source term are proposed in §3. Experimental works are briefly discussed in §4. The governing equations of shear flows rewritten in polar coordinates, and the corresponding initial and boundary conditions are presented in §§5, 6. The numerical results are discussed in §7. In appendix A, the numerical scheme for the non-conservative equations is written in polar coordinates.

## 2. Two-dimensional (2D) shear shallow water flows

The system describing multi-dimensional shear shallow water flows over a flat bottom without friction effects can be written in the form (Teshukov 2007; Gavriluk & Gouin 2012; Richard & Gavriluk 2012, 2013)

$$\left. \begin{aligned} \frac{\partial h}{\partial t} + \operatorname{div}(h\mathbf{U}) &= 0, \\ \frac{\partial(h\mathbf{U})}{\partial t} + \operatorname{div}\left(h\mathbf{U} \otimes \mathbf{U} + \frac{gh^2}{2}\mathbf{I} + h\mathbf{P}\right) &= \mathbf{0}, \\ \frac{D\mathbf{P}}{Dt} + \frac{\partial\mathbf{U}}{\partial\mathbf{x}}\mathbf{P} + \mathbf{P}\left(\frac{\partial\mathbf{U}}{\partial\mathbf{x}}\right)^T &= \mathbf{0}. \end{aligned} \right\} \quad (2.1)$$

The system is obtained by averaging the incompressible Euler equations over the fluid depth and using the hypothesis of smallness of the horizontal vorticity (weakly sheared flows). Here  $t$  is the time,  $\mathbf{x} = (x, y)^T$  are the Cartesian coordinates,  $h$  is the fluid depth,  $g$  is the gravity,  $\mathbf{U} = (U, V)^T$  is the depth-averaged horizontal velocity,  $D/Dt$  means the material derivative with respect to the mean motion

$$\frac{D}{Dt} = \frac{\partial}{\partial t} + \mathbf{U}^T \nabla, \quad (2.2)$$

$\mathbf{P} = \mathbf{P}^T$  is the symmetric stress tensor which measures the distortion of the instantaneous horizontal velocity profile  $\tilde{\mathbf{U}}(t, x, y, z)$ , which depends *a priori* on the vertical coordinate  $z$ . The definitions of  $\mathbf{U}$  and  $\mathbf{P}$  are as follows:

$$\mathbf{U} = \frac{1}{h} \int_0^h \tilde{\mathbf{U}}(t, x, y, z) dz, \quad \mathbf{P} = \frac{1}{h} \int_0^h (\tilde{\mathbf{U}} - \mathbf{U}) \otimes (\tilde{\mathbf{U}} - \mathbf{U}) dz. \quad (2.3a,b)$$

The tensor  $\mathbf{P}$  is positive definite. The positive definiteness of  $\mathbf{P}$  is a consequence of the Cauchy–Schwarz inequality. The sign  $\otimes$  means the tensor product and  $\mathbf{I}$  is the identity tensor. Equations (2.1) admit the energy conservation law

$$\frac{\partial}{\partial t} \left\{ h \left( \frac{1}{2} |\mathbf{U}|^2 + e_i + e_r \right) \right\} + \operatorname{div} \left\{ h\mathbf{U} \left( \frac{1}{2} |\mathbf{U}|^2 + e_i + e_r \right) + \left( \frac{gh^2}{2} \mathbf{I} + h\mathbf{P} \right) \mathbf{U} \right\} = 0, \quad (2.4)$$

with

$$e_i = \frac{1}{2}gh, \quad e_T = \frac{1}{2}\text{tr}(\mathbf{P}), \quad (2.5a,b)$$

and an additional conservation law

$$\frac{\partial h\Psi}{\partial t} + \text{div}(h\mathbf{U}\Psi) = 0, \quad \Psi = \frac{\det(\mathbf{P})}{h^2}. \quad (2.6a,b)$$

The variable  $\Psi$  will be referred to as ‘entropy’ (mathematical) because this quantity is transported along the mean flow in the same way as the true entropy for the Euler equations of compressible fluids. Also, we will see that this quantity will increase across the shocks in analogy with the conventional entropy. The 1D system also admits an analogue of  $\Psi$  having a clear physical meaning. This quantity was associated with the enstrophy (squared vorticity). The enstrophy increase corresponds to vorticity creation: a roller appears at the forward slope of the hydraulic jump (Richard & Gavriluyuk 2012, 2013; Ivanova *et al.* 2017). The system (2.1) is composed of the equations of mass balance, horizontal momentum and the evolution equation for the stress tensor. The governing equations are hyperbolic, if  $\mathbf{P}$  is positive definite, but not in conservative form (for proof, see Gavriluyuk *et al.* (2018a)). For discontinuous solutions, the Rankine–Hugoniot relations come only from the mass, momentum and energy equations. For six unknowns (the fluid depth, two components of the depth-averaged horizontal velocity, and three independent components of the symmetric Reynolds stress tensor) one has only five conservation laws (conservation of mass, momentum, energy and mathematical ‘entropy’). For 1D flows the system of Rankine–Hugoniot relations is closed.

### 3. Dissipative terms compatible with the positive definiteness of the Reynolds stress tensor

Now we add dissipative terms into the model. Even if this process is always empirical, some constraints should be respected. For example, the dissipation law should always be compatible with the energy decrease and guarantee the positive definiteness of the Reynolds stress tensor.

Let us add the dissipative terms in the following form:

$$\left. \begin{aligned} h_t + \text{div}(h\mathbf{U}) &= 0, \\ (h\mathbf{U})_t + \text{div}\left(h\mathbf{U} \otimes \mathbf{U} + \frac{gh^2}{2}\mathbf{I} + h\mathbf{P}\right) &= -C_f|U|U, \\ \frac{D\mathbf{P}}{Dt} + \frac{\partial \mathbf{U}}{\partial \mathbf{x}}\mathbf{P} + \mathbf{P}\left(\frac{\partial \mathbf{U}}{\partial \mathbf{x}}\right)^T &= \mathbf{D}, \quad \mathbf{D} = \mathbf{D}^T. \end{aligned} \right\} \quad (3.1)$$

In particular, the equations for  $\mathbf{P}$  imply

$$h^2 \frac{D}{Dt} \left( \frac{\det(\mathbf{P})}{h^2} \right) = \text{tr}(\mathbf{P})\text{tr}(\mathbf{D}) - \text{tr}(\mathbf{PD}). \quad (3.2)$$

The friction force in the momentum equation is a classical expression, where  $C_f$  is the Chézy coefficient. The dissipative tensor  $\mathbf{D}$  should somehow be defined. The equations (3.1) should satisfy the energy conservation law

$$\begin{aligned} \frac{\partial}{\partial t} \left( h \frac{1}{2} |\mathbf{U}|^2 + e_i + e_T + \operatorname{div} h \mathbf{U} \frac{1}{2} |\mathbf{U}|^2 + e_i + e_T + \frac{gh^2}{2} \mathbf{I} + h \mathbf{P} \right) \mathbf{U} \\ = -C_f |\mathbf{U}|^3 - Q, \end{aligned} \quad (3.3)$$

where the dissipative source term  $Q$  should be positive. The positivity of  $Q$  is the analogue of the second law of thermodynamics. The compatibility of (3.1) and (3.3) implies the expression for  $Q$  in terms of  $\mathbf{D}$ :

$$\operatorname{tr}(\mathbf{D}) = -\frac{2}{h} Q. \quad (3.4)$$

By analogy with the Stokes hypotheses for the Navier–Stokes equations, we assume that the dissipation tensor  $\mathbf{D}$  is an isotropic tensor function of  $\mathbf{P}$ . Then, for the 2D case,  $\mathbf{D}$  is linear in  $\mathbf{P}$ :

$$\mathbf{D} = a\mathbf{P} + b\mathbf{I}, \quad (3.5)$$

where  $a$  and  $b$  are functions of invariants of  $\mathbf{P}$ . Consider the simplest case where  $b=0$ . This choice allows us to obtain a natural reduction to the Saint-Venant equations in the limit  $\mathbf{P}=\mathbf{0}$ , and to interpret the parameter  $-a^{-1} > 0$  as a characteristic relaxation time. Two types of dissipation law will be tested below.

### 3.1. First type of dissipation law

The first type of dissipation is

$$\mathbf{D} = a\mathbf{P}, \quad a = -\frac{2\alpha}{h} |\mathbf{U}|^3, \quad (3.6a,b)$$

where  $\alpha$  has the units  $\text{s}^2 \text{m}^{-2}$ . The multiplier  $-2|\mathbf{U}|^3/h$  is for convenience only. In particular, this choice implies the equation for the ‘entropy’ in the form

$$h^2 \frac{D}{Dt} \left( \frac{\det(\mathbf{P})}{h^2} \right) = -\frac{4\alpha}{h} |\mathbf{U}|^3 \det(\mathbf{P}). \quad (3.7)$$

Equations (3.4) and (3.6) imply the following relation between  $Q$  and  $\alpha$ :

$$Q = \alpha \operatorname{tr}(\mathbf{P}) |\mathbf{U}|^3. \quad (3.8)$$

We will introduce now a new dimensionless parameter  $\kappa = \alpha \operatorname{tr}(\mathbf{P})$ . To recover the 1D case studied previously (Richard & Gavriluk 2012, 2013), we will finally choose

$$\kappa = \max \left( 0, C_r \frac{\frac{\operatorname{tr}(\mathbf{P})}{h^2} - \varphi}{\frac{\operatorname{tr}(\mathbf{P})}{h^2}} \right) \geq 0. \quad (3.9)$$

Here  $\varphi$  and  $C_r$  are the model constants:  $\varphi$  is associated with the enstrophy of small vortices near the bottom, and  $C_r$  is the coefficient associated with the roller dissipation. We will search for these parameters to have qualitative agreement with experiments. As it follows from (3.7) and (3.9), ‘entropy’  $\Psi$  is decreasing on continuous solutions, but always stays positive. This means that the dissipation law also guarantees the positive definiteness of  $\mathbf{P}$ .

### 3.2. Second type of dissipation law

Another type of dissipation tensor  $\mathbf{D}$  can be considered:

$$\mathbf{D} = a\mathbf{P}, \quad a = -\frac{2}{h}\kappa\text{tr}(\mathbf{P})^{1/2}. \quad (3.10a,b)$$

As a consequence of (3.8), one has

$$Q = \kappa\text{tr}(\mathbf{P})^{3/2} > 0. \quad (3.11)$$

This type of dissipation was used, in particular, to study solitary wave breaking (Gavrilyuk *et al.* 2016). Moreover, (3.11) corresponds to the classical energy dissipation rate with the dissipation rate scale  $h$  (Townsend 1956). Such a choice also guarantees that  $\mathbf{P}$  is positive definite for any time, if initially it was positive definite.

## 4. Experiments on hydraulic jumps in a radial flow

Circular hydraulic jumps are commonly observed in a kitchen sink when a vertically falling tap water jet strikes a horizontal plate and then spreads radially outwards (Liu & Lienhard 1993; Bush & Aristoff 2003; Bush *et al.* 2006; Ray & Bhattacharjee 2007; Andersen *et al.* 2010; Eyo *et al.* 2011; Martens *et al.* 2012; Labousse & Bush 2013). Using a more viscous liquid, Ellegaard *et al.* (1998, 1999) and Bush *et al.* (2006) observed breaking of radial symmetry of the hydraulic jump front. More exactly, Ellegaard *et al.* (1998, 1999) obtained polygonal shapes of hydraulic jumps in divergent radial flow with sharp corners: ‘the sharp corners of the polygons carry a large radial flux, while the sides generate resistance to the stream’. Bush *et al.* (2006) also noted that: ‘some polygonal ... forms were subject to weak time-dependent fluctuations, typically characterized by a net rotational motion of the entire jump structure, or the propagation of wave-like disturbances towards a single point on the jump’.

Our numerical study will concern the hydraulic jump in a convergent rather than a divergent radial flow. Such experiments recently performed by Foglizzo *et al.* (2012, 2015). In these experiments, the fluid is radially injected inwards and strikes a hollow cylinder placed in the centre, through which the water is evacuated. As a consequence, a hydraulic jump is formed separating convergent supercritical flow from a deeper subcritical one. The hydraulic jump also exhibits the appearance of radial asymmetries: after formation of the hydraulic jump, it starts to oscillate randomly, then the amplitude of oscillations grows and, finally, the flow becomes very asymmetric and develops a singularity of the jump front that starts to rotate at a constant angular velocity.

We want to understand if model (2.1) is able to reproduce, at least qualitatively, the last experiments. Also, it would be interesting to study the influence of different types of dissipation on the solution properties.

## 5. Governing equations in polar coordinates

To model the hydraulic jumps in radial flow, we rewrite the system (2.1) in polar coordinates (see appendix A). In the dissipationless case over a flat bottom the system (2.1) becomes

$$\frac{\partial(hr)}{\partial t} + \frac{\partial(rhU_r)}{\partial r} + \frac{\partial(hU_\theta)}{\partial \theta} = 0, \quad (5.1)$$

$$\begin{aligned} & \frac{\partial (rhU_r)}{\partial t} + \frac{\partial}{\partial r} \left\{ r \left( hU_r^2 + \frac{gh^2}{2} + hP_{rr} \right) \right\} + \frac{\partial}{\partial \theta} (hU_r U_\theta + hP_{r\theta}) \\ & = h(U_\theta^2 + P_{\theta\theta}) + \frac{gh^2}{2}, \end{aligned} \quad (5.2)$$

$$\begin{aligned} & \frac{\partial (rhU_\theta)}{\partial t} + \frac{\partial}{\partial r} \{ rh (U_r U_\theta + P_{r\theta}) \} + \frac{\partial}{\partial \theta} \left( hU_\theta^2 + \frac{gh^2}{2} + hP_{\theta\theta} \right) \\ & = -h (U_r U_\theta + P_{r\theta}), \end{aligned} \quad (5.3)$$

$$\frac{DP_{rr}}{Dt} - 4\frac{U_\theta}{r}P_{r\theta} + 2 \left( \frac{\partial U_r}{\partial r}P_{rr} + \frac{P_{r\theta}}{r} \frac{\partial U_r}{\partial \theta} \right) = 0, \quad (5.4)$$

$$\frac{DP_{r\theta}}{Dt} + \frac{U_\theta}{r} (P_{rr} - 2P_{\theta\theta}) + \frac{P_{\theta\theta}}{r} \frac{\partial U_r}{\partial \theta} + P_{rr} \frac{\partial U_\theta}{\partial r} + \frac{P_{r\theta}}{r} \left\{ \frac{\partial U_\theta}{\partial \theta} + \frac{\partial (rU_r)}{\partial r} \right\} = 0, \quad (5.5)$$

$$\frac{DP_{\theta\theta}}{Dt} + 2\frac{U_\theta}{r}P_{r\theta} + 2 \left\{ \frac{\partial U_\theta}{\partial r}P_{r\theta} + \frac{P_{\theta\theta}}{r} \left( \frac{\partial U_\theta}{\partial \theta} + U_r \right) \right\} = 0, \quad (5.6)$$

$$\begin{aligned} & \left. \frac{\partial}{\partial t} \left\{ hr \left( \frac{1}{2}|U|^2 + E \right) \right\} + \frac{\partial}{\partial r} \left\{ r \left[ hU_r \left( \frac{1}{2}|U|^2 + E \right) + \frac{gh^2}{2}U_r + h(P_{rr}U_r + P_{r\theta}U_\theta) \right] \right\} \right. \\ & \left. + \frac{\partial}{\partial \theta} \left\{ hU_\theta \left( \frac{1}{2}|U|^2 + E \right) + \frac{gh^2}{2}U_\theta + h(P_{r\theta}U_r + P_{\theta\theta}U_\theta) \right\} = 0, \end{aligned} \quad (5.7)$$

with

$$\frac{D}{Dt} = \frac{\partial}{\partial t} + U_r \frac{\partial}{\partial r} + \frac{U_\theta}{r} \frac{\partial}{\partial \theta}. \quad (5.8)$$

The system (5.1)–(5.7) admits the ‘entropy’ conservation law

$$\frac{D}{Dt} \left( \frac{\det(\mathbf{P})}{h^2} \right) = 0. \quad (5.9)$$

We have now to add into the model the dissipative terms and bottom topography. The bottom will consist of an inclined plane of a mild slope superposed with a bump modelling the hollow cylinder (see figure 1). Analytically, the expression of the bottom function is given by

$$b(r) = \begin{cases} A \frac{[(r - R^- - L_1)^2 - L_1^2]^2}{L_1^4}, & \text{if } 0 < r - R^- < 2L_1, \\ (r - R^- - 2L_1) \tan \beta, & \text{if } 2L_1 + R^- < r < R^+. \end{cases} \quad (5.10)$$

Here  $\beta$  is a small inclination angle,  $A$  is the bump amplitude,  $2L_1$  is the radial bump width,  $R^-$  is the internal boundary radius and  $R^+$  is the external boundary radius. The formula (5.10) is used to model the ‘physical’ cylinder of height  $A$  which is placed at  $r = R^- + L_1$ .

## 6. Initial and boundary conditions

Now, we need to impose boundary conditions on the cylinder. Formally, the number of boundary conditions should be equal to the number of characteristics entering the flow domain. For stationary flows it is natural to impose the criticality



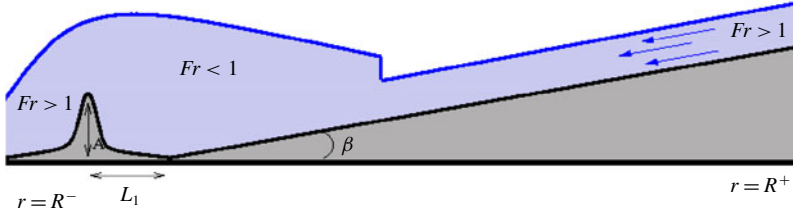


FIGURE 1. (Colour online) The bottom topography used in numerical simulations.

condition (Froude number is equal to one) on the top of the cylinder. When the flow is non-stationary and transcritical, the problem of boundary conditions is an open question even for the Saint-Venant equations (cf. Shiue *et al.* (2011)). Indeed, at a given time we do not know whether the flow is subcritical or supercritical. In particular, if the flow is subcritical, the perturbations can propagate inside the flow domain and change the solution. We thus need a remedy to impose mathematically relevant boundary conditions. For this, we will compute the solution in a larger domain by introducing a bump allowing us to accelerate the downslope flow and transform it from subcritical to supercritical flow. Thus, the flow will be supercritical at both boundaries  $r = R^-$  and  $r = R^+$ . In particular, at  $r = R^-$  there is no characteristic entering the flow domain, so we can use Neumann boundary conditions. At  $r = R^+$  we impose all unknowns. Such an approach allowing us to control the fluid flow at the downstream crest was also used by Winters (2016).

We numerically solved the equations (5.7) where, additionally, the dissipative terms and bottom topography are added (see appendix A) in the computational domain:

$$R^- \leq r \leq R^+, \quad 0 \leq \theta \leq 2\pi. \quad (6.1a,b)$$

The initial conditions are

$$h(r, \theta, t = 0) = \begin{cases} 1.5h_0 \text{ [m]}, & \text{if } r - R^- \leq 0.3(R^+ - R^-), \\ h_0 \text{ [m]}, & \text{else} \end{cases} \quad (6.2)$$

$$U_r(r, \theta, t = 0) = -q_0/(rh), \quad U_\theta(r, \theta, t = 0) = 0, \quad (6.3a,b)$$

$$P_{rr}(r, \theta, t = 0) = \varphi h^2, \quad P_{r\theta}(r, \theta, t = 0) = 0, \quad P_{\theta\theta}(r, \theta, t = 0) = \epsilon \varphi h^2, \quad \epsilon = 10^{-8}. \quad (6.4a-d)$$

Here  $h_0$  is a flow depth at  $r = R^+$ ,  $q_0 > 0$  is a flow discharge. The values of parameters are given in table 1. Obviously, we respect the inequality  $2L_1 < 0.3(R^+ - R^-)$ . We have to underline that the initial conditions are unimportant, because for large time the solution is defined only by the boundary conditions.

This initial radial flow is perturbed at the boundary  $r = R^+$ :

$$h = h_0, \quad U_r = -q_0(1 + 0.01 \sin(n\theta))/(R^+ h_0), \quad U_\theta = 0, \quad P_{rr} = P_{\theta\theta} = 0.5\varphi h_0^2, \quad P_{r\theta} = 0. \quad (6.5a-e)$$

We added a small perturbation of the radial velocity in the  $\theta$ -direction, with  $n$  being any natural number. We took  $n = 16$ , which corresponds to the number of injecting pumps used, in particular, in experiments by Foglizzo *et al.* (2012). The periodic boundary condition is used in the  $\theta$ -direction. We control that at the inner boundary

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$h_0$ [m]	$\beta$ [rad]	$C_r$	$C_f$	$\varphi$ [s <sup>-2</sup> ]	$q_0$ [m <sup>3</sup> s <sup>-1</sup> ]	$R^+$ [m]	$R^-$ [m]	$A$ [m]	$L_1$ [m]
0.003	0.07	0.3–1.0	0.0036	2–20	$1.2 \times 10^{-3}$	1.0	0.08	0.005	0.006

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TABLE 1. Parameters for numerical tests.

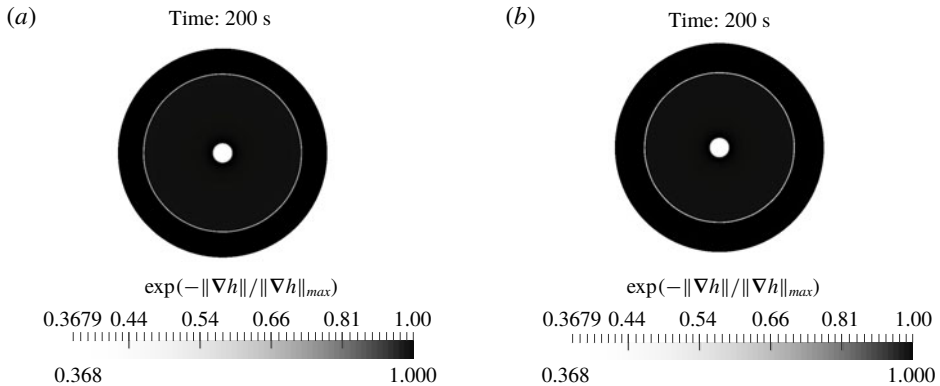


FIGURE 2. (Colour online) Schlieren images of the fluid depth for the Saint-Venant equations are shown (a) with quadratic friction ( $\mathbf{F} = -C_f|\mathbf{U}|\mathbf{U}$ ,  $C_f = 0.0036$ ,  $\beta = 0.05$  [rad]), and (b) with linear friction ( $\mathbf{F} = -(v/h)\mathbf{U}$ ,  $\beta = 0.05$  [rad]), with kinematic viscosity  $\nu = 10^{-6}$  [m<sup>2</sup> s<sup>-2</sup>]. The white boundaries inside the domain correspond to strong gradients of  $h$ . The central white part corresponds to the hole. The ‘schlieren’ function was used in a standard form:  $\exp(-\|\nabla h\|/\|\nabla h\|_{max})$ , where  $\|\cdot\|$  means the Euclidean norm. The same ‘schlieren’ function was used for all computations. The first-order Godunov method with the HLLC Riemann solver was used with  $300 \times 300$  mesh cells. In both cases, the hydraulic jump was stationary. The hydraulic jump radius with the quadratic friction is a little bit larger than that with the linear friction.

$r = R^-$  in the radial direction the corresponding generalized Froude number is larger than one to guarantee the condition of supercritical flow:  $F_g = U_r/\sqrt{gh + 3P_{rr}} > 1$ .

The numerical scheme is based on the splitting technique developed in Gavrilyuk *et al.* (2018a). The first-order Godunov scheme with the HLLC Riemann solver has been used (for details, see appendix A). A mesh convergence study has been performed in order to guarantee a convergent solution. The code has been parallelized using the Message Passing Interface (MPI).

## 7. Numerical results

### 7.1. Saint-Venant equations

The numerical resolution of the classical shallow water equations (Saint-Venant equations) with quadratic and linear friction gives us only stationary hydraulic jumps without any oscillations or formation of rotating patterns (see figure 2). The hydraulic jump radius for the quadratic friction is a little bit larger than that corresponding to the linear friction. The same type of discharge perturbation was used as in (6.5) with the same bottom topography (5.10).

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	$\varphi = 2 \text{ s}^{-2}$	$\varphi = 5 \text{ s}^{-2}$	$\varphi = 10 \text{ s}^{-2}$	$\varphi = 20 \text{ s}^{-2}$
$C_r = 1$	+	+	$\pm$	-
$C_r = 0.5$	+	+	$\pm$	-
$C_r = 0.3$	+	+	$\pm$	-

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TABLE 2. In this table, ‘+’ means that for corresponding values of  $\varphi$  and  $C_r$ , the rotating cusp is formed, ‘-’ means that the cusp is absent. ‘ $\pm$ ’ means the limit behaviour.

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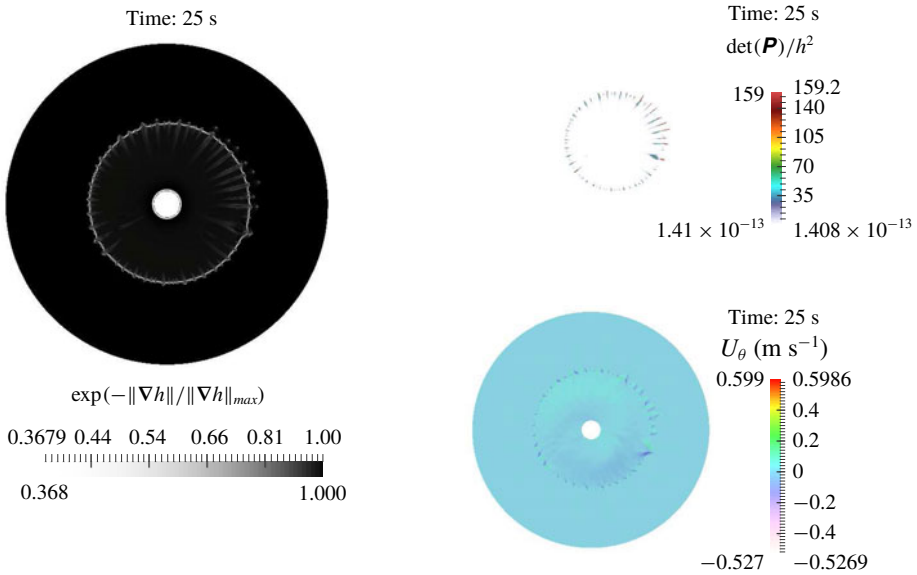


FIGURE 3. (Colour online) The schlieren images of  $h$ ,  $U_\theta$  and  $\Psi = \det(\mathbf{P})/h^2$  are shown. The first-order Godunov method with the HLLC Riemann solver is used with  $500 \times 500$  mesh cells,  $C_r = 1$ ,  $\varphi = 2 \text{ s}^{-2}$ . The central white part corresponds to the hole. The white line in the internal area corresponds to the hydraulic jump position.

### 7.2. Shear shallow water model with the first type of dissipation

We present now the numerical results for the first type of dissipation given by (3.6). The scenario of the hydraulic jump formation in convergent radial flow consists of the following three different stages. First, almost radially symmetric hydraulic jump is formed at a time instant of approximately 25 s (see figure 3). One can see only small-amplitude non-stationary transverse perturbations on the front of the hydraulic jump. This is clearly visible in figure 3 showing, in particular, the distribution of  $\Psi = \det(\mathbf{P})/h^2$  and almost vanishing tangential velocity. Then, the hydraulic jump is destabilized into a pattern exhibiting sloshing-type oscillations at a time instant of approximately 50 s. The oscillation period is approximately 4 s. In figure 4 the schlieren images are shown at time instants 50 s and 52 s (after the half-period of oscillations). Remarkably, in a certain domain of parameters, as the amplitude of the oscillations grows, a new free-surface pattern appears. More exactly, a singular point (cusp) at the free surface of the hydraulic jump appears which starts to rotate at a constant velocity. In figure 5, the cusp rotating in the positive direction is denoted

by  $S$ . As usual, we define the positive direction as a counterclockwise rotation, and negative as a clockwise rotation. The sense of rotation can easily be understood from figure 5. Indeed, ahead of the cusp the front is smooth, and behind the cusp the front is rough: transverse ‘triangular’ structures appear. The formation of ‘triangles’ behind the moving cusp was also experimentally observed (Foglizzo *et al.* 2012). The sense of rotation can be changed by a small initial perturbation. If the initial condition for the tangential velocity is taken in the form  $U_\theta = \pm\mu r$  ( $\mu$  is a small positive parameter), the sense of rotation will be determined by the sign of  $U_\theta$ . The radius of the hydraulic jump increases with time. One can also see the emergence of double jump structures appearing at time instant 150 s, where the free surface changes abruptly (figure 5). The double jump structures were also observed in the case of divergent circular hydraulic jumps in Bush *et al.* (2006).

The cusp with a rotation period of approximately 6 s appears only in a certain region of parameters  $\varphi$  and  $C_r$  (see table 2). The rotation period is insensitive to the variation of  $\varphi$  and  $C_r$ , when they vary in the intervals  $[2, 20]$  [ $\text{s}^{-2}$ ], and  $[0.3, 1]$ , respectively. It has been experimentally proven in Foglizzo *et al.* (2012) (see their figure 3) that the period of rotation of the cusp is proportional to the ‘hydraulic jump radius’, which is the distance between the origin and the cusp’s position. In this sense we are in a good agreement with experimental data, because the period of rotation of our cusp is 6 s for the hydraulic jump radius of approximately 35 cm.

Interestingly enough, when one plots the time evolution of the velocity (both radial and tangential components) at a given point behind (but not too far) the hydraulic jump, it also shows an oscillatory behaviour with the same period of oscillations of approximately 6 s (see figure 6). Figure 6(a) proves the fact that the generalized Froude number is always smaller than one, i.e. at this point the non-stationary flow is always subcritical. Analogous plots for the computations with the Saint-Venant equations with a linear friction show negligible time variations of the flow velocity (figure 7).

In the experiments of Foglizzo *et al.* (2012) the period was approximately 3 s, but the geometry was different; as a consequence the radius of the hydraulic jump and its period of rotation were different. The main reason why we did not use the same geometry was that we could not guarantee supercritical flow at the outlet (at  $r = R^-$ ), which was necessary to assure the correct boundary conditions for the governing equations.

When  $\varphi$  vanishes, we do not obtain any transverse structures: the circular hydraulic jump only is formed. Recall that  $\varphi$  is associated with the enstrophy of small vortexes near the bottom Richard & Gavriluk (2012, 2013).

In table 2 we studied how enstrophy  $\varphi$  and the parameter  $C_r$  influence the flow behaviour. Numerical tests showed that the solution is more sensitive to the variation of  $\varphi$  than of  $C_r$ . For  $\varphi$  larger than  $10 \text{ s}^{-2}$  the cusp disappears, but a singular point is still visible (see figure 8).

A numerical ‘cusp’ solution was also obtained in Foglizzo *et al.* (2012) by using the Saint-Venant equations with linear friction, but for a different bottom topography. They used the criticality condition at the top of the cylinder, which corresponds to the hypothesis of the steady flow assumption.

A surprising fact also mentioned in the experiments is that the sense of rotation of the cusp and the tangential fluid velocity near the hole is opposite. This fact is also described by our model.

Our mathematical model of shear shallow water flows and the corresponding numerical method are thus capable of simulating new flow patterns appearing in radial fluid flows.

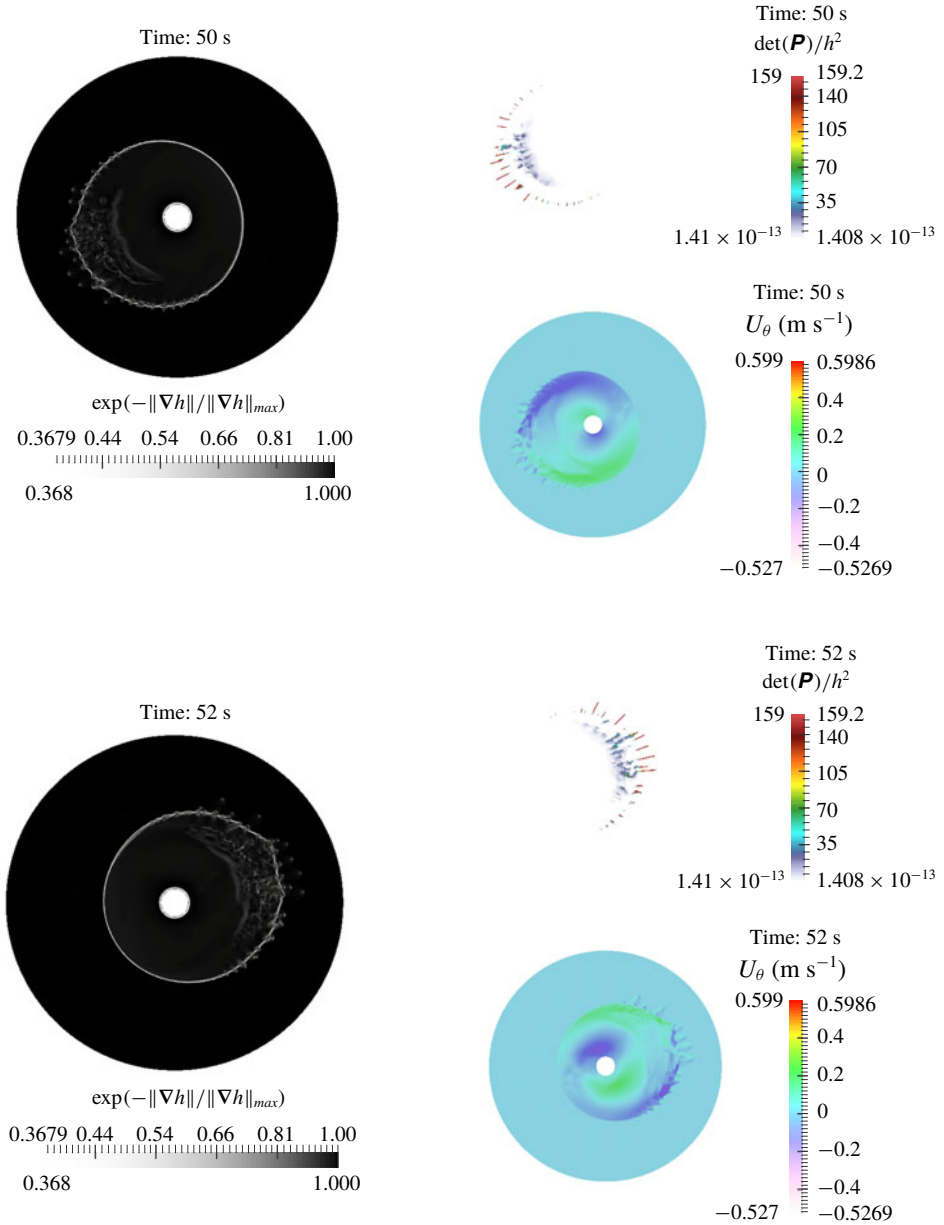


FIGURE 4. (Colour online) The schlieren images of  $h$ ,  $U_\theta$  and  $\Psi = \det(\mathbf{P})/h^2$  are shown. The first-order Godunov method with the HLLC Riemann solver is used with  $500 \times 500$  mesh cells,  $C_r = 1$ ,  $\varphi = 2 \text{ s}^{-2}$ . The hydraulic jump is destabilized into a pattern which corresponds to ‘sloshing’-type oscillations.

### 7.3. Shear shallow water model with the second type of dissipation

Using the second type of dissipative terms (3.10), one can see the formation of a new flow pattern: a rotating square-like hydraulic jump appears (see figure 9). The ‘corners’ of the ‘square’ are rather smooth and the shape between ‘corners’ is convex, while the

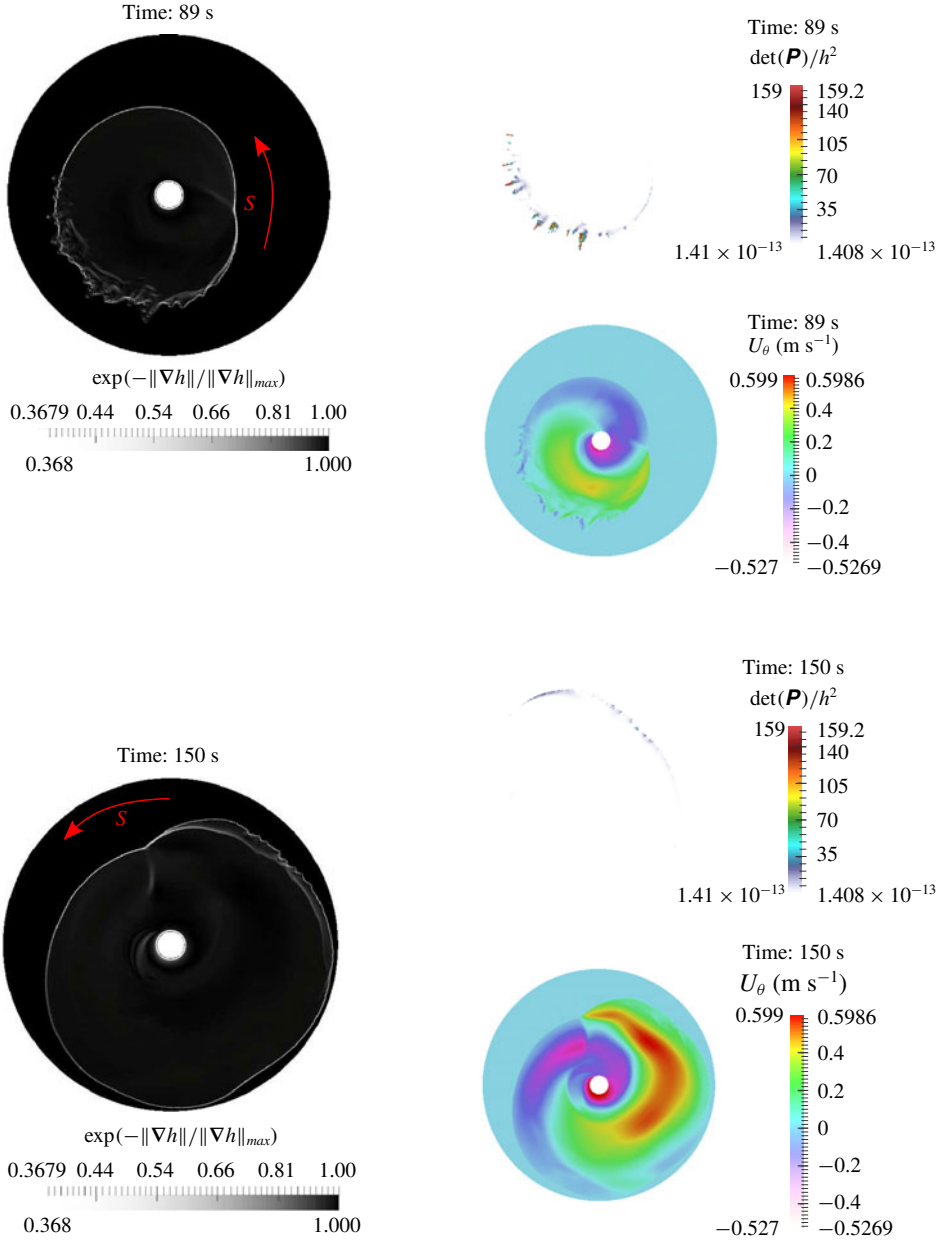


FIGURE 5. (Colour online) The schlieren images of  $h$ ,  $U_\theta$  and  $\Psi = \det(\mathbf{P})/h^2$  are shown. The first-order Godunov method with the HLLC Riemann solver is used with  $500 \times 500$  mesh cells,  $C_r = 1$ ,  $\varphi = 2 \text{ s}^{-2}$ . The cusp (a singular point) denoted by  $S$  appears at the front. It rotates in the positive direction (counterclockwise direction) shown by an arrow. Ahead of the cusp, the front is smooth, while behind the cusp the front is rough: transverse triangular structures follow the cusp.

corners were sharp and the shape between corners was concave in the experiments on divergent hydraulic jumps Ellegaard *et al.* (1998). This is the main difference between convergent and divergent radial flows. This type of result is not yet experimentally

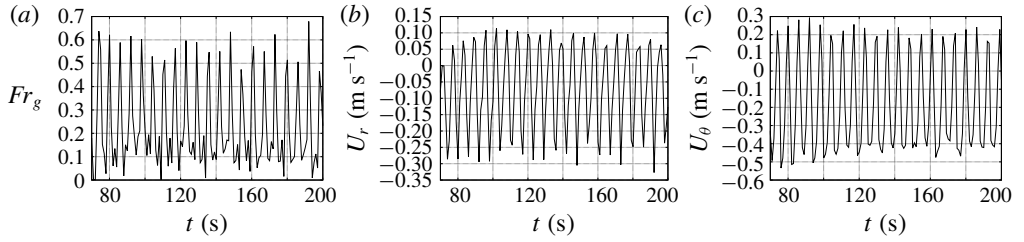


FIGURE 6. The generalized Froude number  $Fr_g = U_r/\sqrt{gh + 3P_{rr}}$  (a) and velocity behaviour (b,c) are shown at the point  $r = 0.5(R^+ - R^-)$ ,  $\theta = 0$  as functions of time. After approximately 70 s, when the cusp is formed, this point is always in the subcritical region (behind the hydraulic jump). As previously,  $C_r = 1$ ,  $\varphi = 2 \text{ s}^{-2}$ . The oscillations are almost periodic with a period of approximately 6 s that corresponds to the cusp rotation period.

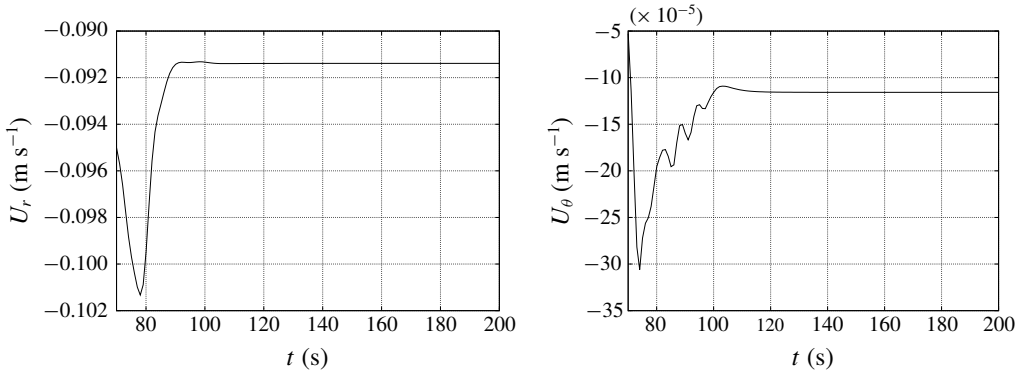


FIGURE 7. The radial and tangential velocity time behaviour at a given point  $r = 0.5(R^+ - R^-)$ ,  $\theta = 0$  are shown after 70 s for the classical Saint-Venant equations with linear friction. This point is always in the subcritical region. The hydraulic jump is an almost stationary one (the variation of  $U_\theta$  is of the order of  $10^{-4} \text{ m s}^{-1}$ ). The quadratic friction force gives the same results.

confirmed. Probably, as in the case of divergent hydraulic jumps, such a ‘rotating square-like structure’ can be obtained by changing the viscosity of the liquids used in the experiments.

## 8. Conclusion

Numerical modelling of multi-dimensional turbulent hydraulic jumps formed in convergent radial flow is performed and qualitatively compared to the experimental observations. Two types of ‘well posed’ dissipation laws are proposed and studied. For the first type of dissipation, in some region of parameters  $\varphi$  and  $C_r$ , the formation of a rotating cusp (angular point) on the hydraulic jump front was found. For the second type of dissipation, rotating square-like hydraulic jumps were found.

The numerical validations show the capability of the model and numerical method to reproduce the multi-dimensional hydraulic jump dynamics.

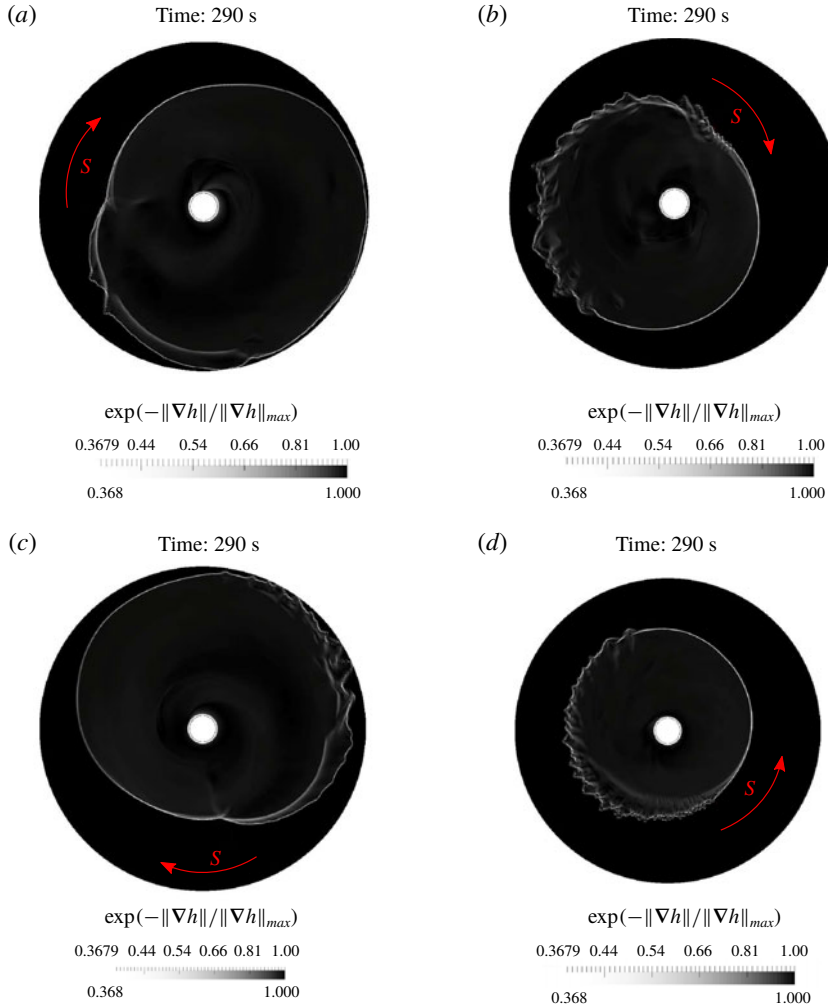


FIGURE 8. (Colour online) Schlieren images of the hydraulic jump corresponding to  $\varphi = 2 \text{ s}^{-2}$  (a),  $\varphi = 5 \text{ s}^{-2}$  (b),  $\varphi = 10 \text{ s}^{-2}$  (c) and  $\varphi = 20 \text{ s}^{-2}$  (d) are shown with  $500 \times 500$  mesh cells at the time instant 290 s. The value of  $C_r$  is taken as 0.5 in each case. The cusp is formed only for  $\varphi = 2 \text{ s}^{-2}$  and  $\varphi = 5 \text{ s}^{-2}$ . In the cases  $\varphi = 10 \text{ s}^{-2}$  and  $\varphi = 20 \text{ s}^{-2}$  the cusp degenerates into a moving singular point through which we have always the transition from the ‘smooth’ to the ‘rough’ region. The corresponding singular point (denoted by  $S$ ) rotates in a negative sense (clockwise direction) for all cases, excepting the case  $\varphi = 20 \text{ s}^{-2}$  where the sense of rotation is positive (counterclockwise direction). The sense of rotation can easily be understood, because ahead of the singular point the front is smooth, while behind the singular point the front is rough.

Further development of this multi-dimensional model would be to add dispersive effects to describe the multi-dimensional wave propagation and breaking as was performed in the 1D case in Gavrilyuk *et al.* (2016) and Gavrilyuk, Liapidevskii & Chesnokov (2018b).



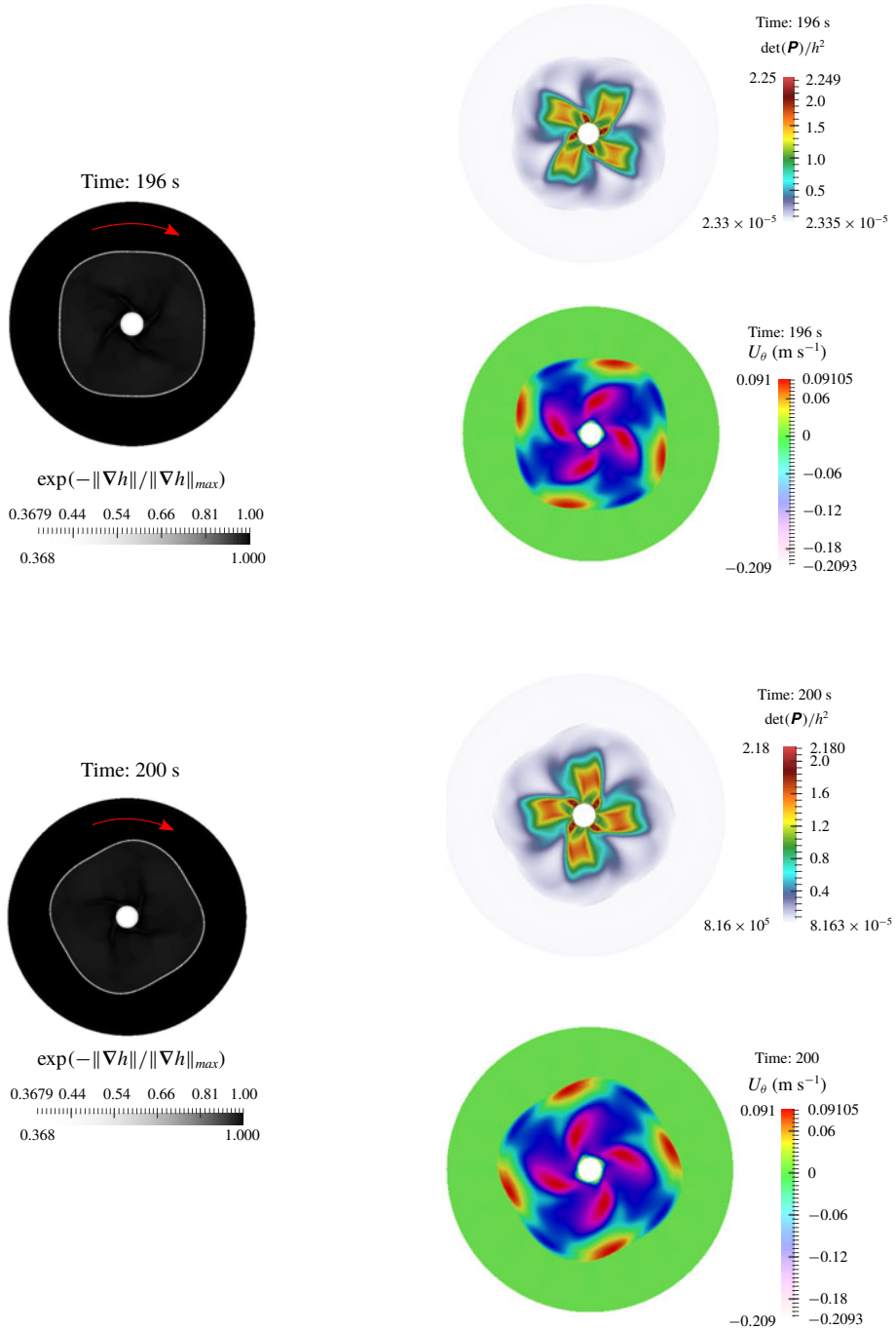


FIGURE 9. (Colour online) The second type of dissipative term is used here. The schlieren images of  $h$ ,  $U_\theta$  and  $\Psi = \det(\mathbf{P})/h^2$  are shown at time instants 196 s and 200 s. The first-order Godunov method with the HLLC Riemann solver was used with  $300 \times 300$  mesh cells,  $C_r = 10$ ,  $\varphi = 100$  [s<sup>-2</sup>],  $q = 10^{-3}$  [m<sup>3</sup> s<sup>-1</sup>]. One can see the appearance of square-like structures with ‘smoothed’ edges which rotate in the clockwise direction.

## Acknowledgements

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## Appendix A

### A.1. Governing equations in polar coordinates

We will rewrite now the system of governing equations (2.1) in polar coordinates  $(r, \theta)$ . We introduce the corresponding basis vectors:

$$\mathbf{e}_r = (\cos \theta, \sin \theta)^\top, \quad \mathbf{e}_\theta = (-\sin \theta, \cos \theta)^\top. \quad (\text{A } 1a,b)$$

One has

$$\frac{\partial \mathbf{e}_r}{\partial r} = 0, \quad \frac{\partial \mathbf{e}_\theta}{\partial r} = 0, \quad \frac{\partial \mathbf{e}_\theta}{\partial \theta} = -\mathbf{e}_r, \quad \frac{\partial \mathbf{e}_r}{\partial \theta} = \mathbf{e}_\theta. \quad (\text{A } 2a-d)$$

One can then use the decomposition

$$\mathbf{U} = U_r \mathbf{e}_r + U_\theta \mathbf{e}_\theta, \quad \frac{\partial \mathbf{U}}{\partial \mathbf{x}} = \frac{\partial \mathbf{U}}{\partial r} \otimes \mathbf{e}_r + \frac{1}{r} \frac{\partial \mathbf{U}}{\partial \theta} \otimes \mathbf{e}_\theta. \quad (\text{A } 3a,b)$$

Using (A 2), (A 3), we obtain

$$\frac{\partial \mathbf{U}}{\partial \mathbf{x}} = \frac{\partial U_r}{\partial r} \mathbf{e}_r \otimes \mathbf{e}_r + \frac{\partial U_\theta}{\partial r} \mathbf{e}_\theta \otimes \mathbf{e}_r + \left( \frac{1}{r} \frac{\partial U_r}{\partial \theta} - \frac{U_\theta}{r} \right) \mathbf{e}_r \otimes \mathbf{e}_\theta + \frac{1}{r} \left( U_r + \frac{\partial U_\theta}{\partial \theta} \right) \mathbf{e}_\theta \otimes \mathbf{e}_\theta. \quad (\text{A } 4)$$

Equation (A 4) implies

$$\operatorname{div} \mathbf{U} = \frac{1}{r} \frac{\partial U_\theta}{\partial \theta} + \frac{1}{r} \frac{\partial}{\partial r} (r U_r). \quad (\text{A } 5)$$

Also, one has

$$\mathbf{P} = P_{rr} \mathbf{e}_r \otimes \mathbf{e}_r + P_{r\theta} \mathbf{e}_r \otimes \mathbf{e}_\theta + P_{\theta r} \mathbf{e}_\theta \otimes \mathbf{e}_r + P_{\theta\theta} \mathbf{e}_\theta \otimes \mathbf{e}_\theta, \quad P_{r\theta} = P_{\theta r}. \quad (\text{A } 6)$$

Then,

$$\operatorname{div}(h\mathbf{P}) = (\operatorname{div}(\mathbf{P}))h + \mathbf{P}\nabla h, \quad (\text{A } 7)$$

where

$$\operatorname{div}(\mathbf{P}) = \left( \frac{\partial P_{rr}}{\partial r} + \frac{1}{r} \frac{\partial P_{r\theta}}{\partial \theta} + \frac{1}{r} (P_{rr} - P_{\theta\theta}), \frac{\partial P_{r\theta}}{\partial r} + \frac{1}{r} \frac{\partial P_{\theta\theta}}{\partial \theta} + \frac{2}{r} P_{r\theta} \right). \quad (\text{A } 8)$$

Also, the material derivative defined by (2.2) is transformed to

$$\frac{\mathbf{D}}{\mathbf{D}t} = \frac{\partial}{\partial t} + U_r \frac{\partial}{\partial r} + \frac{U_\theta}{r} \frac{\partial}{\partial \theta}. \quad (\text{A } 9)$$

Using (A 2), one obtains

$$\frac{\partial \mathbf{P}}{\partial r} = \begin{pmatrix} \frac{\partial P_{rr}}{\partial r} & \frac{\partial P_{r\theta}}{\partial r} \\ \frac{\partial P_{r\theta}}{\partial r} & \frac{\partial P_{\theta\theta}}{\partial r} \end{pmatrix}, \quad \frac{\partial \mathbf{P}}{\partial \theta} = \begin{pmatrix} \frac{\partial P_{rr}}{\partial \theta} - 2P_{r\theta} & \frac{\partial P_{r\theta}}{\partial \theta} + P_{rr} - P_{\theta\theta} \\ \frac{\partial P_{r\theta}}{\partial \theta} + P_{rr} - P_{\theta\theta} & \frac{\partial P_{\theta\theta}}{\partial \theta} + 2P_{r\theta} \end{pmatrix}. \quad (\text{A } 10a,b)$$

One can finally obtain the system (2.1) in polar coordinates:

$$\frac{\partial(hr)}{\partial t} + \frac{\partial(rhU_r)}{\partial r} + \frac{\partial(hU_\theta)}{\partial \theta} = 0, \quad (\text{A } 11\text{a})$$

$$\begin{aligned} \frac{\partial(rhU_r)}{\partial t} + \frac{\partial}{\partial r} \left\{ r \left( hU_r^2 + \frac{gh^2}{2} + hP_{rr} \right) \right\} + \frac{\partial}{\partial \theta} (hU_r U_\theta + hP_{r\theta}) \\ = h(U_\theta^2 + P_{\theta\theta}) + \frac{gh^2}{2}, \end{aligned} \quad (\text{A } 11\text{b})$$

$$\begin{aligned} \frac{\partial(rhU_\theta)}{\partial t} + \frac{\partial}{\partial r} \{ rh(U_r U_\theta + P_{r\theta}) \} + \frac{\partial}{\partial \theta} \left( hU_\theta^2 + \frac{gh^2}{2} + hP_{\theta\theta} \right) \\ = -h(U_r U_\theta + P_{r\theta}), \end{aligned} \quad (\text{A } 11\text{c})$$

$$\frac{DP_{rr}}{Dt} - 4\frac{U_\theta}{r}P_{r\theta} + 2 \left( \frac{\partial U_r}{\partial r}P_{rr} + \frac{P_{r\theta}}{r} \frac{\partial U_r}{\partial \theta} \right) = 0, \quad (\text{A } 11\text{d})$$

$$\begin{aligned} \frac{DP_{r\theta}}{Dt} + \frac{U_\theta}{r} (P_{rr} - 2P_{\theta\theta}) + \frac{P_{\theta\theta}}{r} \frac{\partial U_r}{\partial \theta} + P_{rr} \frac{\partial U_\theta}{\partial r} + \frac{P_{r\theta}}{r} \left\{ \frac{\partial U_\theta}{\partial \theta} + \frac{\partial(rU_r)}{\partial r} \right\} = 0, \\ (\text{A } 11\text{e}) \end{aligned}$$

$$\frac{DP_{\theta\theta}}{Dt} + 2\frac{U_\theta}{r}P_{r\theta} + 2 \left\{ \frac{\partial U_\theta}{\partial r}P_{r\theta} + \frac{P_{\theta\theta}}{r} \left( \frac{\partial U_\theta}{\partial \theta} + U_r \right) \right\} = 0, \quad (\text{A } 11\text{f})$$

$$\begin{aligned} \frac{\partial}{\partial t} \left\{ hr \left( \frac{1}{2}|U|^2 + E \right) \right\} + \frac{\partial}{\partial r} \left\{ r \left[ hU_r \left( \frac{1}{2}|U|^2 + E \right) + \frac{gh^2}{2}U_r + h(P_{rr}U_r + P_{r\theta}U_\theta) \right] \right\} \\ + \frac{\partial}{\partial \theta} \left\{ hU_\theta \left( \frac{1}{2}|U|^2 + E \right) + \frac{gh^2}{2}U_\theta + h(P_{r\theta}U_r + P_{\theta\theta}U_\theta) \right\} = 0. \end{aligned} \quad (\text{A } 11\text{g})$$

## A.2. Numerical splitting technique in polar coordinates

The method follows the splitting technique proposed in Gavriilyuk *et al.* (2018a) in Cartesian coordinates. We adapt this approach to polar coordinates. For this, we solve first the model in the  $r$ -direction, then in the  $\theta$ -direction. For each direction, the two subsystems, proposed below, are solved separately. Each subsystem is hyperbolic and admits the energy conservation law.

### A.2.1. Subsystem 1 in the $r$ -direction

The first subsystem in the  $r$ -direction is

$$\frac{\partial(rh)}{\partial t} + \frac{\partial(rhU_r)}{\partial r} = 0, \quad (\text{A } 12\text{a})$$

$$\frac{\partial(rhU_r)}{\partial t} + \frac{\partial}{\partial r} \left( r \left( hU_r^2 + \frac{gh^2}{2} + hP_{rr} \right) \right) = h(U_\theta^2 + P_{\theta\theta}) + \frac{gh^2}{2}, \quad (\text{A } 12\text{b})$$

$$\frac{\partial(rhU_\theta)}{\partial t} + \frac{\partial}{\partial r} (rhU_r U_\theta) = -h(U_r U_\theta + P_{r\theta}), \quad (\text{A } 12\text{c})$$

$$\frac{\partial(rhP_{rr})}{\partial t} + \frac{\partial(rhU_r P_{rr})}{\partial r} + 2rhP_{rr} \frac{\partial U_r}{\partial r} = 4hP_{r\theta}U_\theta, \quad (\text{A } 12\text{d})$$

$$\frac{\partial(rP_{r\theta})}{\partial t} + \frac{\partial(rU_r P_{r\theta})}{\partial r} = U_\theta (2P_{\theta\theta} - P_{rr}), \quad (\text{A } 12\text{e})$$

$$\frac{\partial(hrP_{\theta\theta})}{\partial t} + \frac{\partial(rhU_r P_{\theta\theta})}{\partial r} = -2h(P_{r\theta}U_\theta + P_{\theta\theta}U_r), \quad (\text{A } 12\text{f})$$

The subsystem (A 12) admits the energy conservation law

$$\frac{\partial}{\partial t} \left\{ rh \left( \frac{1}{2} |U|^2 + E \right) \right\} + \frac{\partial}{\partial r} \left\{ r \left[ hU_r \left( \frac{1}{2} |U|^2 + E \right) + \frac{gh^2}{2} U_r + hP_{rr} U_r \right] \right\} = 0, \quad (\text{A } 13)$$

where  $E = (gh + P_{rr} + P_{\theta\theta})/2$ .

The system is hyperbolic with the eigenvalues given by

$$\lambda_{1,2,3,4} = U_r, \quad \lambda_{5,6} = U_r \pm \sqrt{gh + 3P_{rr}}. \quad (\text{A } 14a,b)$$

The equation (A 12d) is not conservative. The jump relation for this equation is not well defined. The value of  $P_{rr}$  will be corrected using the energy equation (A 13). Numerically, we solve first the system (A 12) where the equation (A 12d) is replaced by

$$\frac{\partial(rhP_{rr})}{\partial t} + \frac{\partial(rhU_r P_{rr})}{\partial r} = 0. \quad (\text{A } 15)$$

We update then  $rhP_{rr}$  by using (A 13).

*Godunov-type method and correction of the non-conservative term  $rhP_{rr}$*

Subsystem 1 can be rewritten in the following form:

$$\frac{\partial \mathbf{W}}{\partial t} + \frac{\partial(r\mathbf{F})}{\partial r} = \mathbf{S}^r, \quad (\text{A } 16)$$

where the vector of ‘conservative’ variables  $\mathbf{W}$ , the vector of fluxes  $\mathbf{F}(\mathbf{W})$  and the geometric source term vector  $\mathbf{S}^r(\mathbf{W})$  are

$$\mathbf{W} = (rh, rhU_r, rhU_\theta, rhP_{rr}, rP_{r\theta}, rhP_{\theta\theta}, rh(E + \frac{1}{2}|U|^2))^T, \quad (\text{A } 17)$$

$$\mathbf{F}(\mathbf{W}) = (hU_r, hU_r^2 + p, hU_r U_\theta, hU_r P_{rr}, U_r P_{r\theta}, hU_r P_{\theta\theta}, hU_r (\frac{1}{2}|U| + E) + pU_r)^T, \quad (\text{A } 18)$$

$$\mathbf{S}^r = \left( 0, h(U_\theta^2 + P_{\theta\theta}) + \frac{gh^2}{2}, -h(U_r U_\theta + P_{r\theta}), \right. \\ \left. 4hP_{r\theta} U_\theta, U_\theta(2P_{\theta\theta} - P_{rr}) - P_{r\theta} U_r, -2h(P_{r\theta} U_\theta + P_{\theta\theta} U_r), 0 \right)^T, \quad (\text{A } 19)$$

with

$$p = gh^2/2 + hP_{rr}, \quad E = (gh + P_{rr} + P_{\theta\theta})/2. \quad (\text{A } 20a,b)$$

Let us consider now a fixed grid of size  $\Delta r = r_{i+1/2} - r_{i-1/2}$ , the time increment is defined as  $\Delta t = t^{n+1} - t^n$  which must respect the Courant–Friedrichs–Lewy (CFL) condition. The discrete values of the vector function  $\mathbf{W}$  at  $(r_i, t^n)$  will be denoted by  $\mathbf{W}_i^n$ . Integrating the conservation laws (A 16) on  $[r_{i-1/2}, r_{i+1/2}] \times [t^n, t^{n+1}]$ , one obtains the conservative finite volume Godunov scheme on a fixed grid:

$$\mathbf{W}_i^{n+1} = \mathbf{W}_i^n - \frac{\Delta t}{\Delta r} (r_{i+1/2}(\mathbf{F}_{i+1/2}^{*,n} - \mathbf{S}_i^{r,n}) - r_{i-1/2}(\mathbf{F}_{i-1/2}^{*,n} - \mathbf{S}_i^{r,n})), \quad (\text{A } 21)$$

where  $\mathbf{F}_{i+1/2}^{*,n}$  and  $\mathbf{F}_{i-1/2}^{*,n}$  are the numerical fluxes. They are constant across interfaces between cells during the time step. For computing the fluxes  $\mathbf{F}_{i+1/2}^{*,n} = \mathbf{F}_{i+1/2}^{*,n}(\mathbf{W}_i^n, \mathbf{W}_{i+1}^n)$  and  $\mathbf{F}_{i-1/2}^{*,n} = \mathbf{F}_{i-1/2}^{*,n}(\mathbf{W}_{i-1}^n, \mathbf{W}_i^n)$ , we use approximate solution of the Riemann problem between cells  $i, i+1$  and  $i-1, i$ , respectively.

Using the energy conservative law, the non-conservative term  $rhP_{rr}$  is updated in the following way:

$$(rhP_{rr}) = 2(rhE) - \frac{g(rh)^2}{r} - (rhP_{\theta\theta}). \quad (\text{A } 22)$$

The ‘mathematical’ entropy  $\Psi$  corresponding to the vorticity formation is increasing through the shock at this step (for proof see Gavriluk *et al.* (2018a)).

### A.2.2. Subsystem 2 in the $r$ -direction

The subsystem 2 in the  $r$ -direction is

$$\frac{\partial(rh)}{\partial t} = 0, \quad (\text{A } 23a)$$

$$\frac{\partial(rhU_r)}{\partial t} = 0, \quad (\text{A } 23b)$$

$$\frac{\partial(rhU_\theta)}{\partial t} + \frac{\partial}{\partial r}(rhP_{r\theta}) = 0, \quad (\text{A } 23c)$$

$$\frac{\partial(rhP_{rr})}{\partial t} = 0, \quad (\text{A } 23d)$$

$$\frac{\partial(rP_{r\theta})}{\partial t} + rP_{rr}\frac{\partial U_\theta}{\partial r} = 0, \quad (\text{A } 23e)$$

$$\frac{\partial(rhP_{\theta\theta})}{\partial t} + 2rhP_{r\theta}\frac{\partial U_\theta}{\partial r} = 0. \quad (\text{A } 23f)$$

It admits the energy conservation law

$$\frac{\partial}{\partial t} \left\{ rh \left( \frac{|U|^2}{2} + E \right) \right\} + \frac{\partial}{\partial r} (rhU_\theta P_{r\theta}) = 0. \quad (\text{A } 24)$$

The system is hyperbolic with the eigenvalues given by

$$\lambda_{1,2,3,4} = 0, \quad \lambda_{5,6} = \pm \sqrt{P_{rr}}. \quad (\text{A } 25a,b)$$

Again, system (A 23)–(A 24) is overdetermined. One can note that there are two non-conservative equations. The product  $(rP_{rr}(\partial U_\theta/\partial r))$  in (A 23e) is well defined. It is not the case for the term  $(2rhP_{r\theta}(\partial U_\theta/\partial r))$  in (A 23f). In the following, this equation will be replaced by  $\partial(rhP_{\theta\theta})/\partial t = 0$  at the first step, and then the energy conservation law will be used to update the value of  $P_{\theta\theta}$ .

In particular, the estimation of  $P_{r\theta}$  at time instant  $(t + \Delta t)$  is given by

$$(rP_{r\theta})(r, t + \Delta t) = (rP_{r\theta})(r, t) + (rP_{rr})(r, t) (U_{\theta,i-1/2}^*(t) - U_{\theta,i+1/2}^*(t)) \frac{\Delta t}{\Delta r}. \quad (\text{A } 26)$$

Here  $U_\theta^*$  it is speed at the contact discontinuity. The non-conservative equation for  $P_{r\theta}$  was discretized in the following way:

$$P_{r\theta}(r, t + \Delta t) = P_{r\theta}(r, t) + P_{rr}(r, t) (U_{\theta,i-1/2}^*(t) - U_{\theta,i+1/2}^*(t)) \frac{\Delta t}{\Delta r}. \quad (\text{A } 27)$$

Here  $U_\theta^*$  is the speed of the contact discontinuity. The equation for  $(rhP_{\theta\theta})$  is also non-conservative. This equation is solved numerically first with zero flux, and then the energy conservation law is used to update the value of  $rhP_{\theta\theta}$ :

$$(rhP_{\theta\theta}) = 2(rhE) - \frac{g(rh)^2}{r} - (rhP_{rr}). \quad (\text{A } 28)$$

The ‘mathematical’ entropy  $\Psi$  is conserved at this step.

We will give now a shorter explication of splitting in the  $\theta$ -direction, because it is completely analogous to that in the  $r$ -direction.

### A.2.3. Subsystem 1 in the $\theta$ -direction

The first subsystem in the  $\theta$ -direction is

$$\frac{\partial(rh)}{\partial t} + \frac{\partial(hU_\theta)}{\partial \theta} = 0, \quad (\text{A } 29a)$$

$$\frac{\partial(rhU_r)}{\partial t} + \frac{\partial}{\partial \theta}(hU_r U_\theta) = 0, \quad (\text{A } 29b)$$

$$\frac{\partial(rhU_\theta)}{\partial t} + \frac{\partial}{\partial \theta} \left( hU_\theta^2 + \frac{gh^2}{2} + hP_{\theta\theta} \right) = 0, \quad (\text{A } 29c)$$

$$\frac{\partial(rhP_{rr})}{\partial t} + \frac{\partial(hU_\theta P_{rr})}{\partial \theta} = 0, \quad (\text{A } 29d)$$

$$\frac{\partial(rP_{r\theta})}{\partial t} + \frac{\partial(U_\theta P_{r\theta})}{\partial \theta} = 0, \quad (\text{A } 29e)$$

$$\frac{\partial(rhP_{\theta\theta})}{\partial t} + \frac{\partial(hU_\theta P_{\theta\theta})}{\partial \theta} + 2hP_{\theta\theta} \frac{\partial U_\theta}{\partial \theta} = 0. \quad (\text{A } 29f)$$

The subsystem (A 29) admits the energy conservation law

$$\frac{\partial}{\partial t} \left\{ rh \left( \frac{1}{2} |\mathbf{U}|^2 + E \right) \right\} + \frac{\partial}{\partial \theta} \left\{ hU_\theta \left( \frac{1}{2} |\mathbf{U}|^2 + E \right) + \frac{gh^2}{2} U_\theta + hP_{\theta\theta} U_\theta \right\} = 0. \quad (\text{A } 30)$$

The system is hyperbolic, with the eigenvalues given by

$$\lambda_{1,2,3,4} = \frac{U_\theta}{r}, \quad \lambda_{5,6} = \frac{U_\theta \pm \sqrt{gh + 3P_{\theta\theta}}}{r}. \quad (\text{A } 31a,b)$$

The ‘mathematical’ entropy  $\Psi$  is increasing through the shock at this step.

### A.2.4. Subsystem 2 in the $\theta$ -direction

The subsystem 2 in the  $\theta$ -direction is

$$\frac{\partial(rh)}{\partial t} = 0, \quad (\text{A } 32a)$$

$$\frac{\partial(rhU_r)}{\partial t} + \frac{\partial}{\partial \theta}(hP_{r\theta}) = 0, \quad (\text{A } 32b)$$

$$\frac{\partial(rhU_\theta)}{\partial t} = 0, \quad (\text{A } 32c)$$

$$\frac{\partial(rhP_{rr})}{\partial t} + 2hP_{r\theta} \frac{\partial U_r}{\partial \theta} = 0, \quad (\text{A } 32d)$$

$$\frac{\partial(rP_{r\theta})}{\partial t} + P_{\theta\theta} \frac{\partial U_r}{\partial \theta} = 0, \quad (\text{A } 32e)$$

$$\frac{\partial(rhP_{\theta\theta})}{\partial t} = 0. \quad (\text{A } 32f)$$

The system is hyperbolic with the eigenvalues given by

$$\lambda_{1,2,3,4} = 0, \quad \lambda_{5,6} = \pm \frac{\sqrt{P_{\theta\theta}}}{r}. \quad (\text{A } 33a,b)$$

The subsystem (A 32) admits the energy conservation law

$$\frac{\partial}{\partial t} \left( rh \left( \frac{1}{2} |\mathbf{U}|^2 + E \right) \right) + \frac{\partial}{\partial \theta} (hP_{r\theta} U_r) = 0. \quad (\text{A } 34)$$

The mathematical ‘entropy’  $\Psi$  is conserved at this step.

#### A.2.5. Integration of source terms

The last step is to integrate the system of ordinary differential equations:

$$\frac{d\mathbf{W}}{dt} = \mathbf{S}(\mathbf{W}), \quad (\text{A } 35)$$

with unknown vector

$$\mathbf{W} = \left( rh, rhU_r, rhU_\theta, rhP_{rr}, rP_{r\theta}, rhP_{\theta\theta}, rh \left( E + \frac{1}{2} |\mathbf{U}|^2 \right) \right)^T. \quad (\text{A } 36)$$

The vector of source terms is

$$\begin{aligned} \mathbf{S}(\mathbf{W}) = & \left( 0, -grh \frac{\partial b}{\partial r} - C_f r \sqrt{U_r^2 + U_\theta^2} U_r, -rC_f \sqrt{U_r^2 + U_\theta^2} U_\theta, rhD_{rr}, rD_{r\theta}, rhD_{\theta\theta}, \right. \\ & \left. -grh \frac{\partial b}{\partial r} U_r - rC_f \left( \sqrt{U_r^2 + U_\theta^2} \right)^3 - rQ \right)^T. \end{aligned} \quad (\text{A } 37)$$

The initial condition  $\mathbf{W}|_{t=0}$  is obtained from the previous hyperbolic step after solving the previous four subsystems. The first-order Euler method is used for such an integration.

## REFERENCES

- ANDERSEN, A., BOHR, T. & SCHNIPPER, T. 2010 Separation vortices and pattern formation. *Theor. Comput. Fluid Dyn.* **24**, 329–334.
- BUSH, J. W. M. & ARISTOFF, J. M. 2003 The influence of surface tension on the circular hydraulic jump. *J. Fluid Mech.* **489**, 229–238.
- BUSH, J. W. M., ARISTOFF, J. M. & HOSOI, A. E. 2006 An experimental investigation of the stability of the circular hydraulic jump. *J. Fluid Mech.* **558**, 33–52.
- ELLEGAARD, C., HANSEN, A. E., HAANING, A., HANSEN, K., MARCUSSEN, A., BOHR, T., HANSEN, J. L. & WATANABE, S. 1998 Creating corners in kitchen sinks. *Nature* **392**, 767–768.
- ELLEGAARD, C., HANSEN, A. E., HAANING, A., HANSEN, K., MARCUSSEN, A., BOHR, T., HANSEN, J. L. & WATANABE, S. 1999 Cover illustration: polygonal hydraulic jumps. *Nonlinearity* **12**, 1–7.
- EYO, A. E., JOSHUA, E. E. & UDOH, P. J. 2011 Two dimensional laminar flow of a liquid with circular hydraulic jump. *Mod. Appl. Sci.* **5**, 56–68.
- FOGLIZZO, T., KAZERONI, R., GUILLET, J. & MASSET, F. 2015 The explosion mechanism of core-collapse supernovae: progress in supernova theory and experiments. *Publ. Astron. Soc. Aust.* **32**, e009.
- FOGLIZZO, T., MASSET, F., GUILLET, J. & DURAND, G. 2012 Shallow water analogue of the standing accretion shock instability: experimental demonstration and a two-dimensional model. *Phys. Rev. Lett.* **108**, 051103.
- GAVRILYUK, S. L. & GOUIN, H. 2012 Geometric evolution of the Reynolds stress tensor. *Intl J. Engng Sci.* **59**, 65–73.
- GAVRILYUK, S. L., IVANOVA, K. A. & FAVRIE, N. 2018a Multidimensional shear shallow water flows: problems and solutions. *J. Comput. Phys.* **366**, 252–280.
- GAVRILYUK, S. L., LIAPIDEVSKII, V. YU. & CHESNOKOV, A. A. 2016 Spilling breakers in shallow water: applications to Favre waves and to the shoaling and the breaking of the solitary wave. *J. Fluid Mech.* **808**, 441–468.
- GAVRILYUK, S. L., LIAPIDEVSKII, V. YU. & CHESNOKOV, A. A. 2018b Interaction of a subsurface bubble layer with long internal waves. *Eur. J. Mech. (B/Fluids)*, in press; doi:[10.1016/j.euromechflu.2017.07.004](https://doi.org/10.1016/j.euromechflu.2017.07.004).
- IVANOVA, K. A., GAVRILYUK, S. L., NKONGA, B. & RICHARD, G. L. 2017 Formation and coarsening of roll-waves in shear shallow water flows down an inclined rectangular channel. *Comput. Fluids* **159**, 189–203.
- KASIMOV, A. R. 2008 A stationary circular hydraulic jump, the limits of its existence and its gasdynamic analogue. *J. Fluid Mech.* **601**, 189–198.
- LABOUSSE, M. & BUSH, J. W. M. 2013 The hydraulic bump: the surface signature of a plunging jet. *Phys. Fluids* **25**, 229–238.
- LIU, X. & LIENHARD, J. H. 1993 The hydraulic jump in circular jet impingement and in other thin liquid films. *Exp. Fluids* **15**, 108–116.
- MARTENS, E. A., WATANABE, S. & BOHR, T. 2012 Model for polygonal hydraulic jumps. *Phys. Rev. E* **85**, 036316.
- RAY, A. K. & BHATTACHARJEE, J. K. 2007 Standing and travelling waves in the shallow-water circular hydraulic jump. *Phys. Lett. A* **371**, 241–248.
- RICHARD, G. L. 2013 Elaboration d'un modèle d'écoulements turbulents en faible profondeur: application au ressaut hydraulique et aux trains de rouleaux. PhD thesis, Aix-Marseille.
- RICHARD, G. L. & GAVRILYUK, S. L. 2012 A new model of roll waves: comparison with brocks experiments. *J. Fluid Mech.* **698**, 374–405.
- RICHARD, G. L. & GAVRILYUK, S. L. 2013 The classical hydraulic jump in a model of shear shallow-water flows. *J. Fluid Mech.* **725**, 492–521.
- ROJAS, N., ARGENTINA, M. & TIRAPEGUI, E. 2013 A progressive correction to the circular hydraulic jump scaling. *Phys. Fluids* **25**, 042105.
- SHIUE, M. C., LAMINIE, J., TEMAM, R. & TRIBBA, J. 2011 Boundary value problems for the shallow water equations with topography. *J. Geophys. Res.* **116**, C02015.



- TESHUKOV, V. M. 2007 Gas dynamic analogy for vortex free-boundary flows. *J. Appl. Mech. Tech. Phys.* **48**, 303–309.
- TEYMOURTASH, A. R., KHAVARI, M. & PASSANDIDEH-FARD, M. 2010 Experimental and numerical investigation of circular hydraulic jump. In *Proceedings of the 18th Annual International Conference on Mechanical Engineering* (ed. A. Nouri-Borujerdi & M. R. Movahhedi), ISME2010, vol. 1, p. 35, Paper No. 3537.
- TOWNSEND, A. A. 1956 *The Structure of Turbulent Shear Flow*. Cambridge University Press.
- WINTERS, K. B. 2016 The turbulent transition of a supercritical downslope flow: sensitivity to downstream conditions. *J. Fluid Mech.* **792**, 997–1012.